

Prepared in cooperation with the
OKLAHOMA DEPARTMENT OF TRANSPORTATION

Flood Frequency Estimates and Documented and Potential Extreme Peak Discharges in Oklahoma

Water-Resources Investigations Report 01–4152



Cover: Photograph was taken October 23, 2000, during the Apache, Oklahoma, flood. Photographer: Stanley Wright, The Apache News.



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By Robert L. Tortorelli and Lan P. McCabe

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Conversion Factors and Datum

Multiply	By	To obtain
Length		
inch (in.)	25.4	millimeter (mm)
mile (mi)	1.609	kilometer (km)
Area		
square mile (mi ²)	2.590	square kilometer (km ²)
Flow rate		
cubic foot per second (ft ³ /s)	0.02832	cubic meter per second (m ³ /s)

Vertical coordinate information is referenced to North American Vertical Datum of 1988 (NAVD 88).

Horizontal coordinate information is referenced to North American Datum of 1983 (NAD 83).

Sea level: In this report “sea level” refers to the National Geodetic Vertical Datum of 1929 (NGVD of 1929)—a geodetic datum derived from a general adjustment of the first-order level nets of both the United States and Canada, formerly called Sea Level Datum of 1929.

Flood Frequency Estimates and Documented and Potential Extreme Peak Discharges in Oklahoma

By Robert L. Tortorelli and Lan P. McCabe

Abstract

Knowledge of the magnitude and frequency of floods is required for the safe and economical design of highway bridges, culverts, dams, levees, and other structures on or near streams; and for flood plain management programs. Flood frequency estimates for gaged streamflow sites were updated, documented extreme peak discharges for gaged and miscellaneous measurement sites were tabulated, and potential extreme peak discharges for Oklahoma streamflow sites were estimated. Potential extreme peak discharges, derived from the relation between documented extreme peak discharges and contributing drainage areas, can provide valuable information concerning the maximum peak discharge that could be expected at a stream site. Potential extreme peak discharge is useful in conjunction with flood frequency analysis to give the best evaluation of flood risk at a site.

Peak discharge and flood frequency for selected recurrence intervals from 2 to 500 years were estimated for 352 gaged streamflow sites. Data through 1999 water year were used from streamflow-gaging stations with at least 8 years of record within Oklahoma or about 25 kilometers into the bordering states of Arkansas, Kansas, Missouri, New Mexico, and Texas. These sites were in unregulated basins, and basins affected by regulation, urbanization, and irrigation.

Documented extreme peak discharges and associated data were compiled for 514 sites in and near Oklahoma, 352 with streamflow-gaging stations and 162 at miscellaneous measurements sites or streamflow-gaging stations with short record, with a total of 671 measurements. The sites are fairly well distributed statewide, however many streams, large and small, have never been monitored.

Potential extreme peak-discharge curves were developed for streamflow sites in hydrologic regions of the state based on documented extreme peak discharges and the contributing drainage areas.

Two hydrologic regions, east and west, were defined using 98 degrees 15 minutes longitude as the dividing line.

Introduction

Knowledge of the magnitude and frequency of floods is required for the safe and economical design of highway bridges, culverts, dams, levees, and other structures on or near streams. Flood plain management programs and flood-insurance rates also are based on flood magnitude and frequency information. A flood is any relatively high streamflow overtopping the natural or artificial banks in any reach of a stream (Leopold and Maddock, 1954, p. 249-251). The magnitude of a flood is referred to as the flood peak, which is the highest value of the discharge or stage attained by a flood; thus, peak discharge or peak stage (Langbein and Isseri, 1960, p.10). Three kinds of flood frequency analyses may be conducted; (1) peak discharge; (2) peak stage; and (3) total volume (Dalrymple, 1960, p. 5). Peak-discharge flood frequency analyses are the most common and are the type of flood frequency analyses that will be presented in this investigation.

Documented historical peak-discharge data are valuable for giving perspective to flood potential for local communities near a streamflow-gaging site. Often very large floods happened so long ago that people have forgotten or are unaware that the floods happened and could happen again. These documented peak discharges may be much larger than large damaging streamflows that have recently occurred.

The potential extreme peak discharge at a site, which is an estimate of the maximum expected peak discharge that could occur at a stream site, is used in conjunction with flood frequency analysis to give the best evaluation of flood risk at a site. Extreme flood potential exceeds the discharge associated with large recurrence-interval flood, such as the 100-year peak discharge (Asquith and Slade, 1995). Potential extreme peak-discharge curves, derived from the relation between documented extreme peak-discharge measurements and contributing drainage areas from a hydrologic region, are not associated with specific probabilities or frequencies, but give evidence as to the magnitude of flow that has occurred and can occur. Given similar basin characteristics, a peak lying close to the envelope curve might occur at other basins in the same region (Crippen, 1982). The U. S. Geological Survey (USGS), in cooperation with the Oklahoma Department of Transportation, conducted an investigation to define the potential extreme peak discharges in Oklahoma.

Purpose and Scope

The purpose of this report is to: (1) update flood frequency estimates for gaged streamflow sites with 8 years or more of record for unregulated, regulated, and urban basins in and near Oklahoma, using data through 1999 water year; (2) present documented extreme peak discharges for gaged and miscellaneous measurement sites; (3) present potential extreme peak-discharge curves for unregulated basins for the state; and (4) present potential extreme peak-discharge estimates for all the streamflow measurement sites used in this investigation.

The potential extreme peak-discharge curves were developed based on documented extreme peak-discharge measurements from 352 streamflow-gaging stations in Oklahoma and within about 25 kilometers of Oklahoma in the bordering states of Arkansas, Kansas, Missouri, New Mexico, and Texas (fig. 1; table 1, back of report); and 162 sites in Oklahoma at miscellaneous measurement sites without streamflow-gaging stations, or streamflow-gaging stations with short record (fig. 2; table 2, back of report). The peak-discharge measurements presented are from unregulated basins, and basins affected by regulation, urbanization, and irrigation. An unregulated basin is defined as a drainage basin for which the peak discharges are not affected by regulation, reservoirs, diversions, urbanization, or other human-related activities. Significant regulation by dams or other manmade modification of streamflow is defined as 20 percent or more of the contributing drainage basin being affected (Heimann and Tortorelli, 1988).

This report updates the flood frequencies presented in Heimann and Tortorelli (1988). This update can be used to estimate flood discharges for Oklahoma streamflow-gaging sites with a drainage area greater than 2,510 square miles, because it includes 15 years of additional annual peak data and records from many additional gaging stations, including major peak discharges recorded during 1987, 1990, 1993, and 1995 water years. This report also includes and updates the flood frequencies in Tortorelli (1997), which estimated flood discharges for Oklahoma streamflow-gaging sites with drainage areas less than or equal to 2,510 square miles.

This report also updates the potential extreme peak-discharge analysis by Crippen and Bue (1977) for Oklahoma.

Acknowledgments

The following U.S. Geological Survey personnel provided assistance with this report: Darrell Walters and Tony Coffey provided accurate and valuable information about historic streamflow-gaging data; William Asquith provided guidance about the investigation methodology; and Michael Stallings and Jason Masoner produced the streamflow-gaging station site maps.

Flood Frequency Estimates for Gaged Streamflows

The curvilinear relation between flood peak magnitude and annual exceedance probability or recurrence interval is referred to as a flood frequency curve. Annual exceedance probability is the probability of a given flood magnitude being equaled or exceeded in any one year. Recurrence interval is the reciprocal of the annual exceedance probability, and represents the average number of years between peak flow exceedances of that magnitude. For instance, a flood having an annual exceedance probability of 0.01 has a recurrence interval of 100 years. This does not imply that a 100-year flood peak will be equaled or exceeded each 100 years, but that it will be equaled or exceeded on the *average* of once every 100 years (Thomas and Corley, 1977). That peak might be exceeded in successive years, or more than once in the same year. The probability of that peak happening is called risk. Procedures for making flood risk estimates are given by the Interagency Advisory Committee on Water Data (IACWD) (1982).

The IACWD (1982) provides a standard procedure for flood frequency estimation using the log-Pearson Type III (LPIII) distribution. The procedure uses systematically collected and historical peak-discharge values to define frequency distribution. The shape of the distribution is defined by a skew coefficient used in the estimation procedure.

The LPIII distribution does not always define a suitable distribution of peak-discharge values because of variation in the climatic and physiographic characteristics in the basin. The data distribution is defined by Weibull plotting positions (Chow and others, 1988). An inappropriate fit of the LPIII distribution to the distribution of peak-discharge data can produce erroneous values for flood frequency. Therefore, for the estimation of flood frequency in this investigation, available historical flood information, low-outlier thresholds, and skew coefficients were all considered, following the IACWD guidelines. LPIII flood frequency estimates of the 2-, 5-, 10-, 25-, 50-, 100-, and 500-year floods are given for each gaged station used in this investigation in table 1 (back of report).

Annual Peak Data

All pertinent annual peak-discharge data were collated and reviewed to begin the flood frequency analysis. This review of data eliminated discrepancies across state lines and accounted for data in the immediate bordering areas of a state with similar hydrology.

The station flood frequency analysis presented is based on annual peak-discharge data systematically collected at 352 gag-

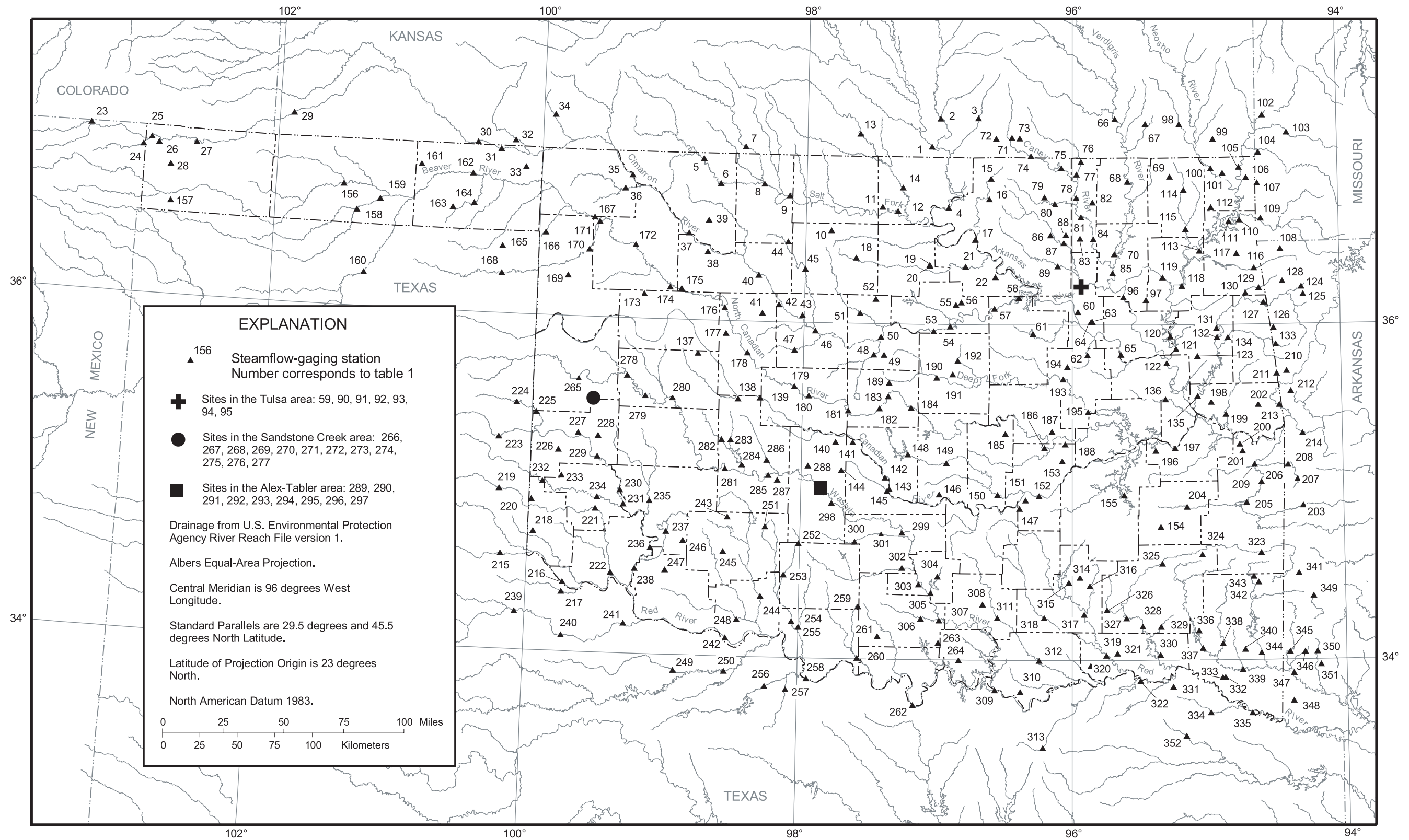


Figure 1. Location of streamflow-gaging stations with at least 8 years of peak discharge data used in study.

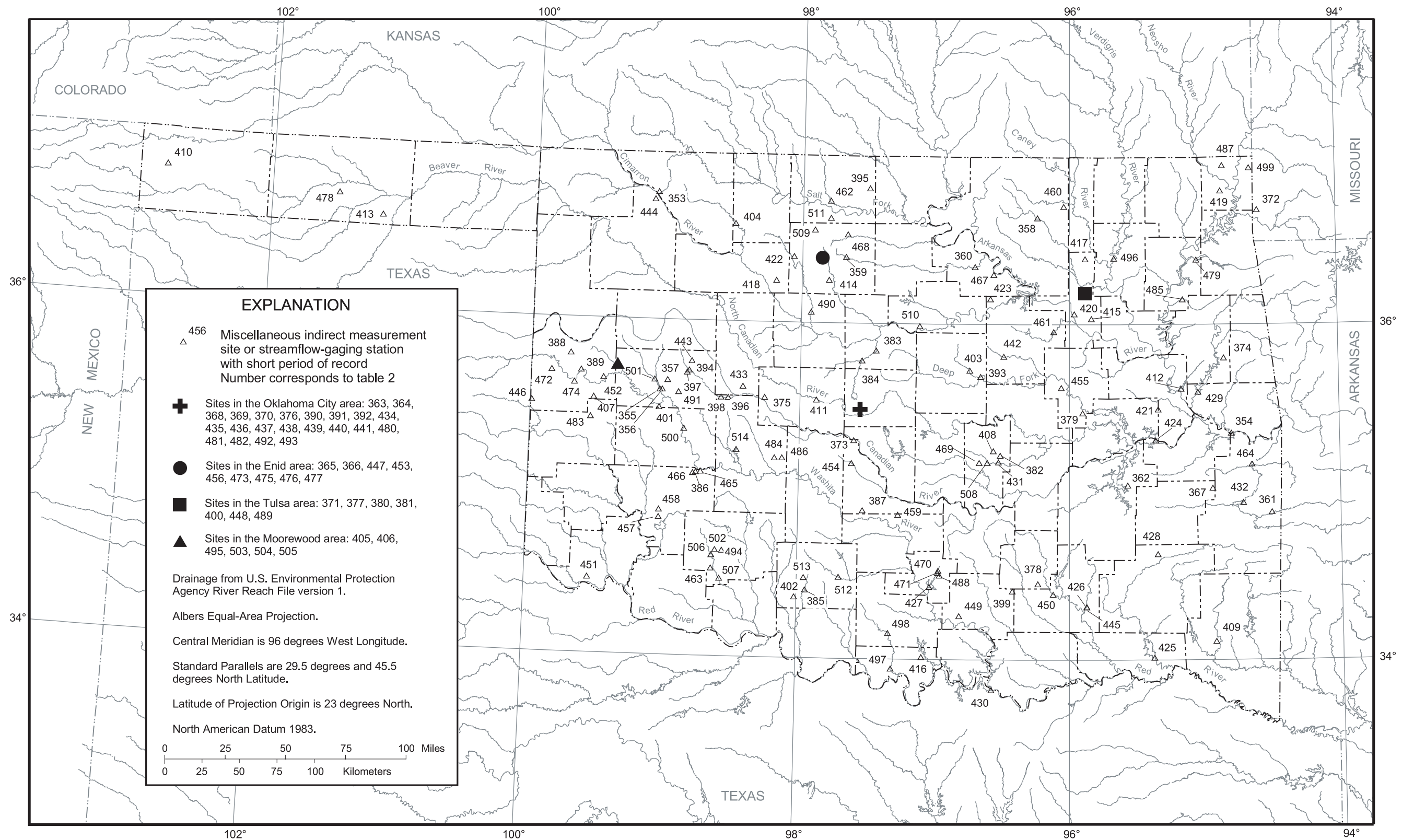


Figure 2. Location of miscellaneous indirect measurement sites and streamflow-gaging stations with short periods of record used in study.

ing stations (fig. 1; table 1, back of report). Those data were based on a water year, October 1 through September 30. Those data were collected through September 30, 1999, for all stations used in this investigation. Only those stations with at least 8 years of flood peak data were used in the analysis. The IACWD (1982) recommends using at least 10 years of data to make these calculations. The only time stations with less than 10 years of data were used was to fill regional gaps; twelve crest-stage partial record sites (sites 16, 40, 79, 110, 117, 144, 198, 200, 218, 247, 314, 324) and eight continuous record sites (sites 61, 136, 165, 191, 283, 285, 291, 342) (fig. 1; table 1, back of report).

All station data were divided into appropriate periods of record, those periods in which the basins were unregulated, and those periods in which there were substantial effects from regulation by major dams or floodwater retarding structures and other manmade modifications. Therefore, each basin condition was analyzed separately if 8 or more years of record were available.

Historical Peak Discharges

In addition to the systematically collected peak-discharge data from gaging stations, the USGS routinely compiles, through newspaper accounts and interviews with local residents, information about historical peak discharges and historical peak stages, so that historical peak elevations can be determined for sites or times without measured data. A historical peak discharge is the highest peak discharge since a known date and may precede the installation of the station; a historical peak discharge can occur either before or after installation of a station. Historical information is critical for evaluating flood frequency estimates for the larger recurrence intervals. Many historical peak discharges are associated with catastrophic storms. Large storms can cause flood peaks exceeding those that can be estimated accurately by analyses of available precipitation or annual peak-discharge data.

Historical peak-discharge data also are valuable for giving perspective to flood potential for local communities near a streamflow-gaging site without the need to attach a statistical meaning to the flood. Often very large peak discharges, both historical peak discharges and systematically collected peak discharges, have occurred so long in the past that people have forgotten or are unaware that the floods have occurred. These peak discharges may be much larger than recent large notable floods. For example, the residents of Blackwell, Oklahoma, experienced a large flood on the Chikaskia River (site 14, fig. 1; table 1, back of report) with a peak discharge of 60,700 cubic feet per second on November 1, 1998, when the river rose about 31 feet in less than two days. However, historic records show that there have been larger peak discharges. The largest is a historical peak discharge of 100,000 cubic feet per second on June 10, 1923, before the streamflow gage was installed. The second largest flood was on June 22, 1942, after the gage was installed, when the peak discharge was 85,000 cubic feet per second,

almost 50 percent more flow than the 1998 flood; three other peak discharges exceeded the 1998 peak discharge.

Historical peak-discharge data are available for over 20 percent of the 352 Oklahoma and border-state stations. These peaks are designated with an "H" in table 1 (back of report). Historical peak discharge is included in frequency estimates by the specifying of a high-outlier threshold and historical record length according to guidelines in the IACWD (1982).

Historical information from nearby streamflow gages was used for a small number of stations, including time of large peaks and period of record. These stations are indicated by the footnotes in table 1 (back of report). For many of these stations, usually those with short periods of record, one gage-recorded peak discharge is historically important because it is considerably greater than the other peak discharges. Although no official documentation of the historical importance of that peak discharge is available, a historical perspective was developed through consideration of a longer period of record from relevant nearby stations. Such consideration was necessary to produce more realistic flood frequency analyses for these stations.

Low-Outlier Thresholds

The climatic and physiographic characteristics of some streams in Oklahoma result in extremely small annual peak-discharge values, referred to as low outliers. Typically, low outliers are identified by visually fitting the data to the LPIII distribution curve. The presence of low outliers can substantially affect the distribution curve; therefore, the fit of the LPIII distribution to the data should be adjusted to account for the presence of low outliers. All peak-discharge values below the low-outlier threshold, including zero, are excluded from the fitting of the LPIII distribution.

The IACWD (1982) guidelines provide a computational procedure for low-outlier threshold selection; however, the IACWD procedure may not produce accurate low-outlier thresholds for some stations. Therefore, the fit of the preliminary LPIII distribution to the distribution of the peak-discharge data for each station was visually inspected and some stations were assigned a revised low-outlier threshold based on that inspection.

Skew Coefficients

The IACWD (1982) guidelines recognize three types of skew coefficients: (1) the station skew coefficient calculated from only the systematic record with appropriate adjustments for high and low outliers, if applicable; (2) the generalized skew coefficient from a locally developed generalized skew map or the IACWD (1982) generalized skew map; and (3) the weighted skew coefficient, calculated by combining the locally developed generalized skew or the IACWD (1982) generalized skew with station skew coefficients.

The station skew coefficient is difficult to estimate reliably for stations with short periods of record. The IACWD (1982)

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recommends applying a weighted skew coefficient to the LPIII distribution. The weighted skew coefficient estimate is calculated by weighting the skew coefficient computed from the peak-discharge data at the station (station skew) and the generalized skew coefficient representative of the surrounding area. A weighted skew coefficient is based on the inverse of the respective mean square errors for each of the station and generalized skew coefficients.

Generalized skew coefficients were determined for Oklahoma (Tortorelli and Bergman, 1985) using adjusted station skew coefficients from stations with at least 20 years of peak-discharge data, streamflow data through 1980, and drainage basin areas greater than 10 square miles and less than or equal to 2,510 square miles. Tortorelli and Bergman (1985) updated the generalized skew coefficients recommended by the IACWD (1982), based on data through 1973. Updating the 1985 Oklahoma generalized skew map was not part of this project. However, a check of the standard error of the generalized skew, using the stations used to develop the generalized skew map and updated streamflow records through 1995, indicated that the standard error value of 0.33 was still valid (Tortorelli, 1997). That standard error value was used to compute weighted skew coefficients using the station and Oklahoma generalized skews for all unregulated basins (designated with a “N” in table 1, back of report) with contributing drainage areas *less than or equal to* 2,510 square miles.

The IACWD (1982) weighted skew coefficients were used for all unregulated basins (designated with a “N” in table 1, back of report) with contributing drainage areas *greater than* 2,510 square miles.

Weighted skew coefficients are not appropriate for stations for which there has been significant effects from regulation by major dams or floodwater retarding structures and other man-made modifications. The station skew coefficient was calculated from only the systematic record with appropriate adjustments for high and low outliers, if applicable, for these types of basins (designated with an “R, U, or I” in table 1, back of report).

Documented Extreme Peak Discharges

The USGS has monitored and published streamflow data for almost 100 years at streamflow-gaging stations throughout Oklahoma, including compilation of annual peak discharges. The USGS also determines peak discharges for large floods at sites without streamflow-gaging stations, through indirect measurements at miscellaneous streamflow measurement sites. Qualifications are assigned to the peak discharges that document the nature of each peak discharge and provide information regarding regulation, reservoirs, land use, and other characteristics affecting the discharge values.

The documented extreme peak discharge was tabulated for each of 352 sites with streamflow-gaging stations (table 1, back of report). The site number, USGS station number, USGS sta-

tion name, type of station, type of record, date and magnitude of the documented extreme peak discharge, magnitude of potential extreme peak discharge (described in next section), contributing drainage area, latitude and longitude of station, hydrologic region, type of basin, and LPIII flood frequency estimates (described in previous section) are presented in table 1. If the documented extreme peak discharge was described in a flood report, that report is noted by a footnote. If a station had more than one type of record, all are presented.

The documented extreme peak discharge also was tabulated at each of 162 selected sites in Oklahoma at miscellaneous measurement sites without streamflow-gaging stations or with streamflow-gaging stations with short periods of record (table 2, back of report). These data were tabulated by visually inspecting the indirect streamflow measurement files at District office. Some have been reported as a historical peak in table 1 and were not repeated in table 2. Many of these peak discharges are associated with catastrophic storms and represent some of the largest peak discharges for the corresponding contributing drainage areas in the state. The descriptive information listed in table 2 is the same as in table 1, except that table 2 lists stream name or indirect measurement site name in place of USGS station name. A USGS station number was noted only on those sites that had a streamflow-gaging station. No LPIII flood frequency estimates were computed. If the documented extreme peak discharge was reported in a flood report, that report is noted by a footnote. If a station had more than one type of record, all are presented.

The sites are fairly well distributed statewide, however many streams, large and small, have never been monitored. The location of each site with streamflow-gaging stations is shown on figure 1. The site numbers on the figure refer to those in table 1, back of report, for sites 1-352. The location of each site without streamflow-gaging stations or streamflow-gaging stations with short periods of record is shown on figure 2. The site numbers on the figure refer to those in table 2, back of report, for sites 353-514. The distribution of the documented peak-discharge measurements from these sites is listed in table 3. A total of 671 streamflow measurements were used from the 514 sites.

Potential Extreme Peak Discharges

The documented extreme peak discharges were analyzed to estimate the potential extreme peak discharges for Oklahoma. Curves enveloping the documented extreme peak discharges for different regions of the state were developed as a function of the corresponding contributing drainage areas of the streamflow measurement sites. The relation between documented extreme peak discharge and other basin characteristics, such as channel length and channel slope, were evaluated by Asquith and Slade (1995). They reported that the potential extreme peak discharge correlates better with contributing drainage areas than with other characteristics. Crippen and Bue (1977) and Paul Jordan (USGS, written commun., 2000) also

Table 3. Summary of drainage area and state distribution of extreme peak discharge measurements

Contributing drainage area (square miles)	Number of extreme peak discharge measurements						Total
	Border states						
	Oklahoma	Arkansas	Kansas	Missouri	New Mexico	Texas	
0.1 to less than 1	22	4	1				27
1 to less than 10	115	2	2			1	120
10 to less than 100	120	9	4	2		2	137
100 to less than 1,000	154	9	9	3	1	13	189
1,000 to less than 10,000	119	3	5	1		11	139
10,000 to less than 50,000	33		2			11	46
50,000 or more	10	3					13
Total	573	30	23	6	1	38	671

report that contributing drainage area is the single most influential basin characteristic to use for determination of potential extreme peak-discharge curves. Therefore, other characteristics were not used in the development of the potential extreme peak-discharge curves for Oklahoma. The envelope curve of discharge data is referred to as potential extreme peak-discharge curve (Asquith and Slade, 1995).

Documented extreme peak discharges 25 kilometers into the bordering states were used to expand the data base of streamflow measurements and to account for data in the immediate bordering areas of a state with similar hydrology. The documented extreme peak discharges were plotted by state to check if the potential extreme peak-discharge curve analysis may be unduly influenced by bordering state data (fig. 3). Only one bordering state data point influenced the analysis, the largest documented extreme peak discharge near Van Buren, Arkansas, (site 214, table 1, back of report), the point at which the Arkansas River flows out of Oklahoma. This point is the upper limit in the east hydrologic region described in succeeding sections.

One possible discriminator for potential extreme peak-discharge curves for the state tested and rejected was dividing the data into the two major drainage basins, the Arkansas River basin and the Red River basin. The documented extreme peak discharges were plotted by major drainage basins (fig. 4) and it was decided by visual inspection that there was not enough difference of discharges between basins to warrant using this criterion. There does not appear to be a meaningful role for statistical testing of documented extreme peak discharges between envelope-curve hydrologic regions (W.F. Kirby, USGS, written

commun., 2001); therefore, no statistical test was performed to verify this conclusion.

Another possible discriminator tested and accepted was dividing the data into two sets, east and west of a line roughly corresponding to the 28-inch mean annual precipitation line (Tortorelli, 1997), which divides the state into an east and west region. The documented extreme peak discharges were plotted by dividing the data into two hydrologic regions, east and west, separated by a longitude line, 98 degrees 15 minutes. It was decided by visual inspection that there was a significant difference of discharges between regions, and again no statistical test was performed to verify this conclusion. This was the criterion that was adopted to define two hydrologic regions. The resulting potential extreme peak-discharge curves are shown in figure 5 for the east region and figure 6 for the west region.

Peak-discharge data from all types of basins are presented in the graphs to see what type of peak-discharge measurement records define the potential extreme peak-discharge curves (figs. 5 and 6). The peak-discharge measurements presented are from unregulated basins and basins affected by regulation, urbanization, and irrigation. All extreme peak-discharge measurements, regardless of basin type, are documented in this publication to see if extreme peak-discharge measurements from other than unregulated basins would control, or define the potential extreme peak-discharge curves.

The relation between the estimated 100-year flood frequency discharge and the contributing drainage area for each of the streamflow-gaging stations was plotted

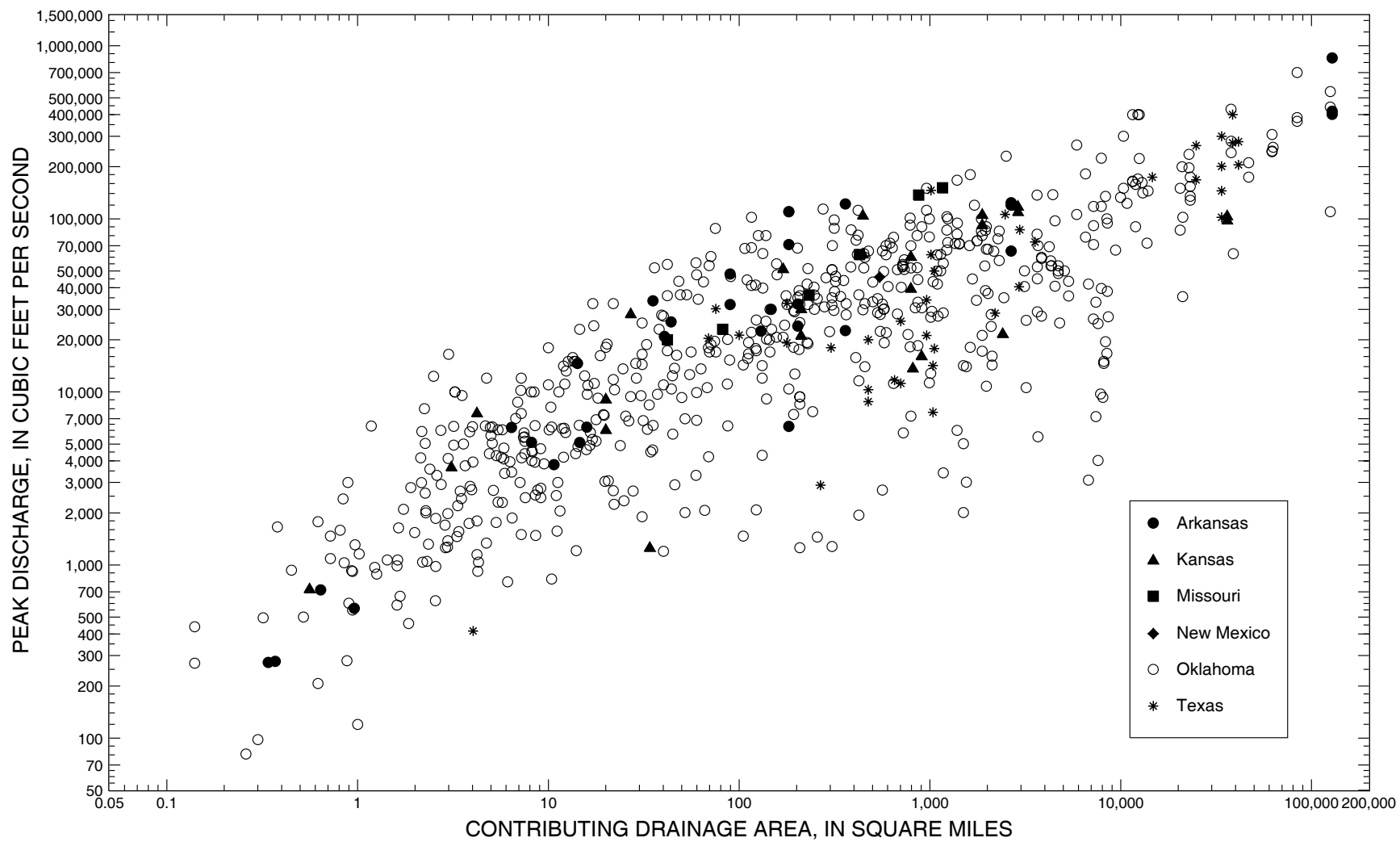


Figure 3. Distribution of extreme peak-discharge data by state.

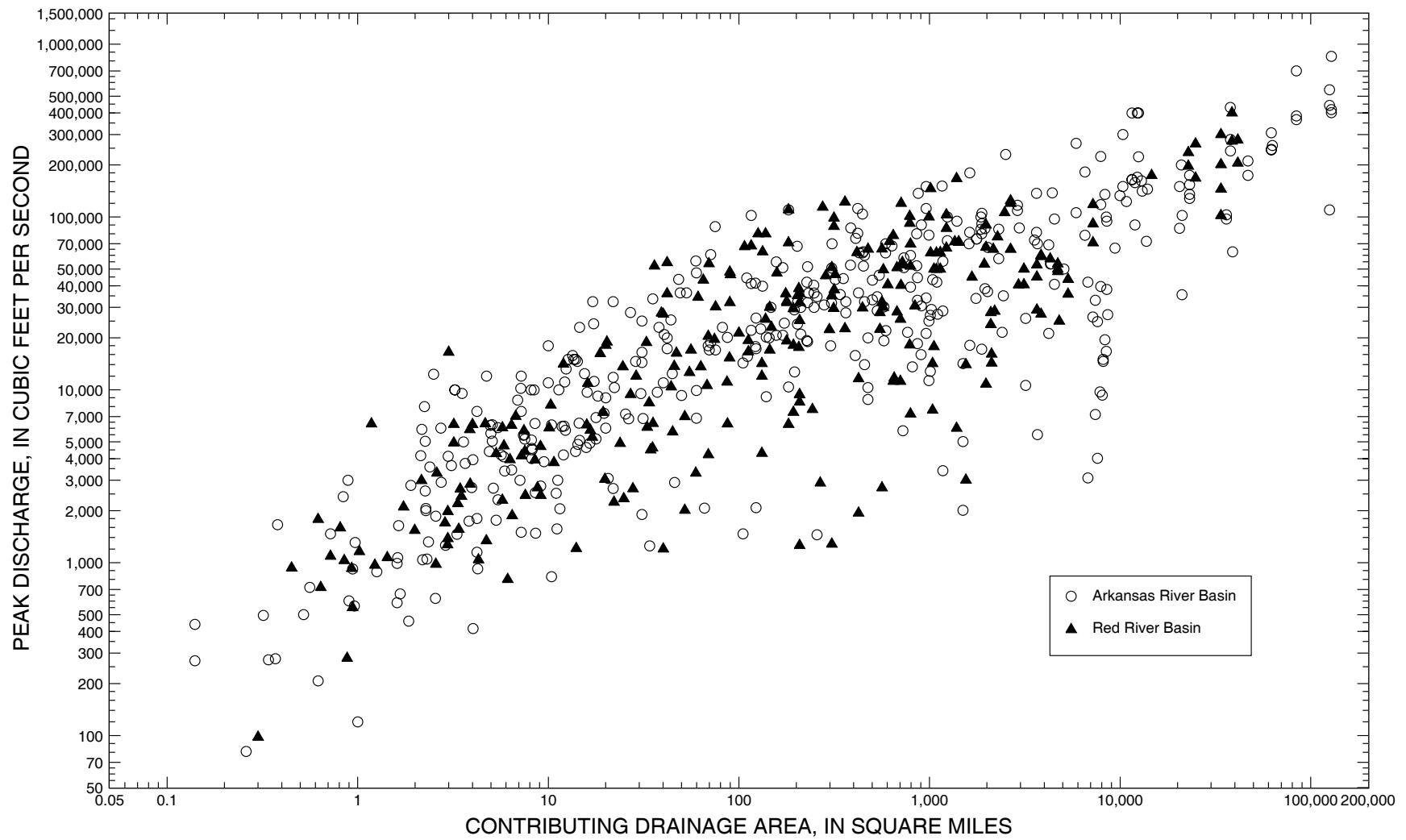


Figure 4. Distribution of extreme peak-discharge data by major drainage basins.

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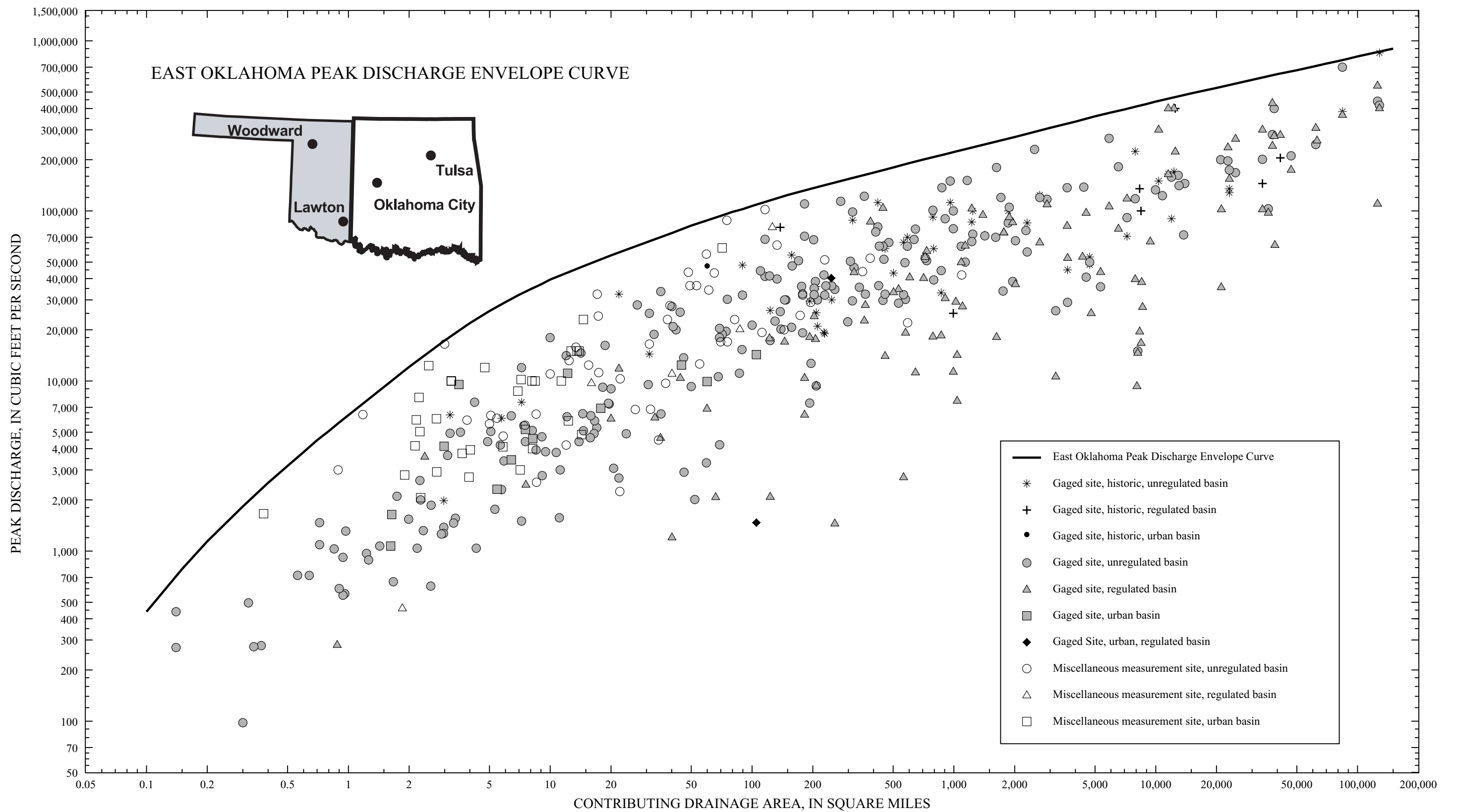


Figure 5. Oklahoma Peak Discharge Envelope Curve based on peak-discharge measurements at streamflow sites east of 98 degree 15 minutes longitude.

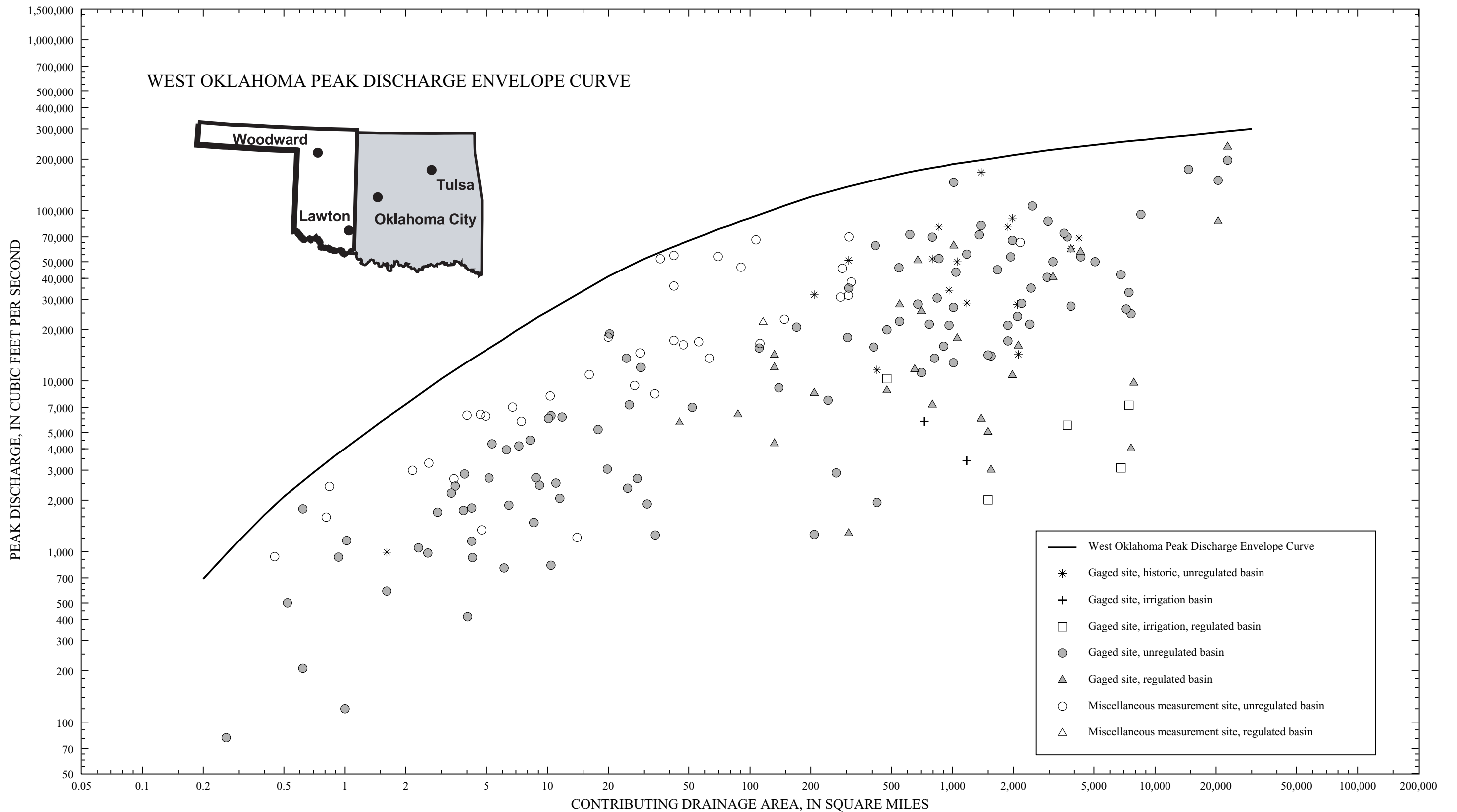


Figure 6. Oklahoma Peak Discharge Envelope Curve based on peak-discharge measurements at streamflow sites west of 98 degrees 15 minutes longitude.

and used to visually check each of the regional potential extreme peak-discharge curves as suggested by Asquith and Slade (1995). The 100-year peak discharges are listed in table 1 (back of report). These data resulted in the slight upward adjustment of both regional curves in the area below 1.0 square mile and above 1,000 square miles.

The potential extreme peak-discharge curves developed used all peak data as of 1999 water year and will be subject to change as greater peak discharges are subsequently documented. The upward trend of the curves through time is probably due to an increased number of streamflow-gaging stations and an increased period of record (Creager, 1939). However, the rate of increase in peak discharges experienced in the United States has been slowing due to a longer period of recorded data and, perhaps, to approaching geophysical limits (Wolman and Costa, 1984; Matthai, 1969). Longer periods of record also would tend to minimize the effect of weather fluctuations.

Generally, the extreme peak-discharge measurements did define the potential extreme peak-discharge curves in figures 5 and 6. Miscellaneous measurements of peak discharge in unregulated basins control the curve for drainage basin areas of about 200 square miles and less for the east region; a few miscellaneous measurements of peak discharge in urban basins control the curve for about 5 square miles and less. Miscellaneous measurements of peak discharge in unregulated basins control the curve for drainage basin areas of about 1,000 square miles and less for the west region. The potential extreme peak-discharge curve is defined mostly by measurements of peak discharge in unregulated basins at streamflow-gaging stations in the east region and a few measurements of peak discharge in regulated basins at streamflow-gaging stations and historical peaks, for drainage areas greater than 200 square miles (fig. 5). The potential extreme peak-discharge curve is defined by measurements of peak discharge in unregulated basins at streamflow-gaging stations and historical peaks in the west region for drainage areas greater than 1,000 square miles (fig. 6). One measurement from a regulated basin in the east region was used, Red River near Terral, Okla. (site 258, fig. 1; table 1, back of report), in the west region curve. That measurement was used to provide a reasonable upper limit for the curve since most of the drainage area for the site is in the west region. A comparison of the potential extreme peak-discharge curves for two hydrologic regions (figs. 5 and 6) is shown in figure 7.

A potential extreme peak-discharge estimate for any site in a unregulated basin can be obtained from the potential extreme peak-discharge curve for the hydrologic region containing the site, if the contributing

drainage area is known. Since all types of drainage basins were used to develop the curves, extreme peak-discharge estimates for sites in which there have been significant effects from manmade modification of streamflow may be obtained if caution is exercised to recognize the limitations of such estimates. For example, streams regulated by major dams are subject to reservoir operations. Urban basins with a high percentage of impervious land cover such as concrete, asphalt and buildings, when coupled with a highly localized storm, could conceivably have higher peak flow. Potential extreme peak-discharge estimates of all 514 sites are listed in tables 1 and 2 (back of report). The curves are presented in tabular form for convenience (table 4). Recurrence intervals cannot be associated with potential extreme peak-discharge estimates because the discharge data do not meet the criteria for statistical analysis (P.R. Jordan, USGS, written commun., 2001).

Summary

Knowledge of the magnitude and frequency of floods is required for the safe and economical design of highway bridges, culverts, dams, levees, and other structures on or near streams; and for flood plain management programs. The potential extreme peak discharge at a site, which is an estimate of the maximum expected peak discharge that could occur at a stream site, often is used in conjunction with flood frequency analysis to give the best evaluation of flood risk at a site. Potential extreme peak-discharge curves, derived from the relation between documented extreme peak-discharge measurements and the contributing drainage areas from a hydrologic region, are not associated with specific probabilities or frequencies, but give evidence as to the magnitude of flow that has occurred.

This report: (1) updates flood frequency estimates for gaged streamflow sites with 8 years or more of record for unregulated, regulated, and urban basins in and near Oklahoma, using data through 1999 water year; (2) presents documented extreme peak discharges for gaged and miscellaneous measurement sites; (3) presents potential extreme peak-discharge curves for unregulated basins for the State; and (4) presents potential extreme peak-discharge estimates for all the streamflow measurement sites used in this investigation.

Peak discharge and flood frequency for selected recurrence intervals from 2 to 500 years were determined for 352 gaged streamflow sites. Data through 1999 water year were used from streamflow-gaging stations with at least 8 years

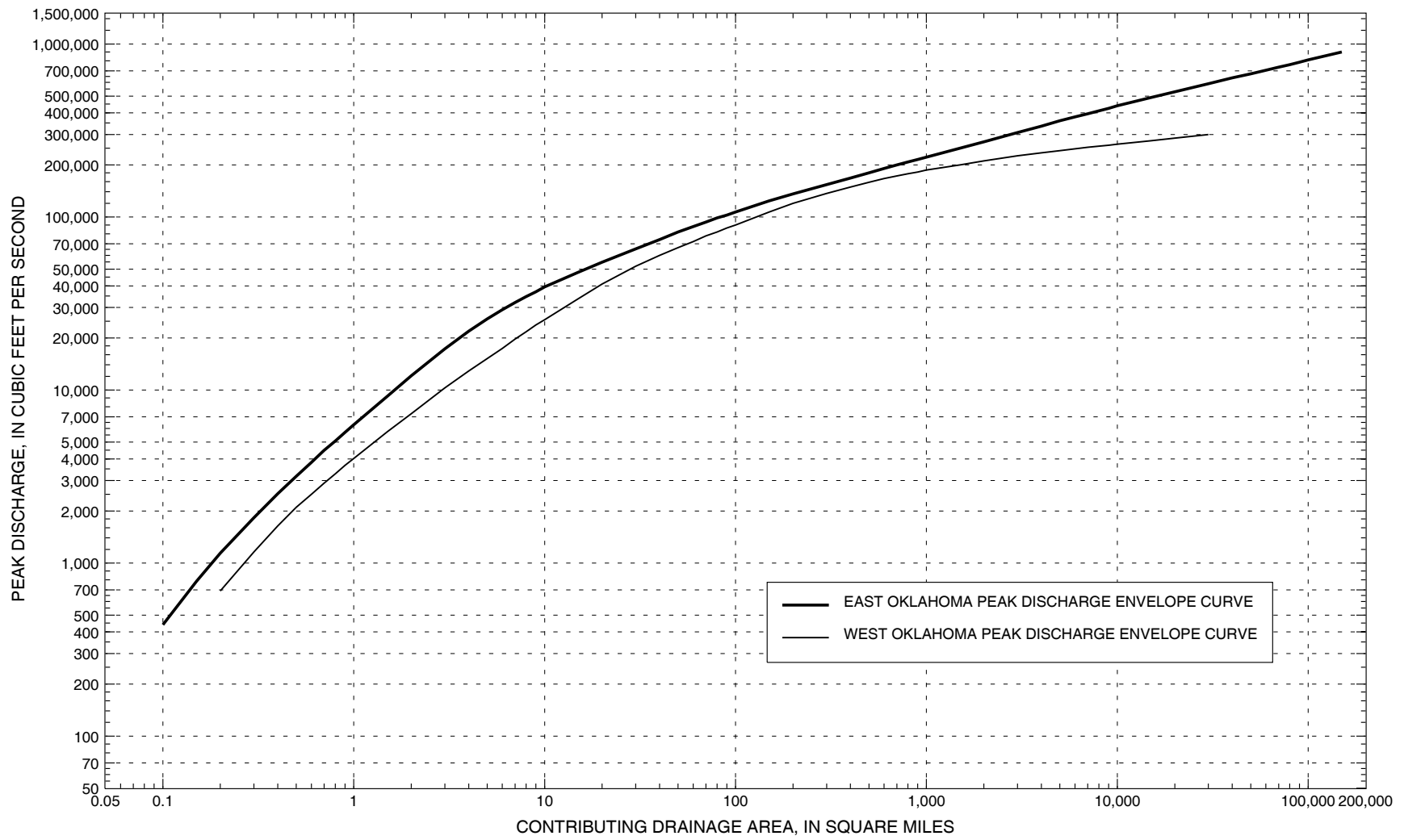


Figure 7. Comparison of East and West Oklahoma Peak Discharge Envelope Curves.

Table 4. Oklahoma Peak Discharge Envelope Curve Data[mi², square miles; East, sites east of 98 degrees 15 minutes longitude; West, sites west of 98 degrees 15 minutes longitude]

Contributing drainage area (mi ²)	Peak discharge		Contributing drainage area (mi ²)	Peak discharge (cubic feet per second)	
	East	West		East	West
0.1	440		100	107,000	90,000
0.15	785		150	124,000	107,000
0.2	1,140	690	200	136,000	120,000
0.3	1,830	1,160	300	154,000	137,000
0.4	2,520	1,640	400	168,000	149,000
0.5	3,170	2,100	500	180,000	159,000
0.6	3,820	2,500	600	191,000	167,000
0.7	4,490	2,900	700	200,000	173,000
0.8	5,080	3,280	800	208,000	178,000
0.9	5,700	3,670	900	215,000	182,000
1	6,300	4,020	1,000	222,000	187,000
1.5	9,220	5,750	1,500	250,000	200,000
2	12,100	7,300	2,000	272,000	211,000
3	17,300	10,300	3,000	308,000	226,000
4	21,900	12,900	4,000	335,000	235,000
5	25,800	15,200	5,000	360,000	242,000
6	29,100	17,400	6,000	379,000	248,000
7	32,100	19,700	7,000	395,000	253,000
8	34,700	21,700	8,000	411,000	257,000
9	37,000	23,800	9,000	425,000	260,000
10	39,500	25,500	10,000	440,000	264,000
15	47,900	33,700	15,000	491,000	276,000
20	54,800	41,000	20,000	529,000	286,000
30	65,400	52,000	30,000	590,000	300,000
40	74,100	60,000	40,000	637,000	
50	82,000	66,500	50,000	672,000	
60	88,000	72,100	60,000	705,000	
70	93,500	77,800	70,000	735,000	
80	98,800	82,000	80,000	760,000	
90	102,500	86,500	90,000	785,000	
			100,000	810,000	
			150,000	900,000	

of record within Oklahoma or about 25 kilometers into the bordering states of Arkansas, Kansas, Missouri, New Mexico, and Texas. These sites were in unregulated basins, and basins affected by regulation, urbanization, and irrigation.

Two types of documented extreme peak discharges are presented. These are maximum peak discharges documented at 352 sites with streamflow-gaging stations within and near Oklahoma and selected large peak discharges documented at 162 selected sites in Oklahoma at miscellaneous measurement sites without streamflow-gaging stations or streamflow-gaging stations with short record, with a total of 671 measurements. The sites are fairly well distributed statewide, however many streams, large and small, have never been monitored.

Potential extreme peak-discharge curves were developed for streamflow sites in hydrologic regions of the state based on documented extreme peak discharges and the contributing drainage areas. Two hydrologic regions, east and west, were defined, using 98 degrees 15 minutes longitude as the dividing line. The relation between the estimated 100-year flood frequency peak discharge and the contributing drainage area for each of the streamflow-gaging stations also was used to check and adjust each of the regional potential extreme peak-discharge curves.

A potential extreme peak-discharge estimate for any site in a unregulated basin can be obtained from the potential extreme peak-discharge curve for the hydrologic region containing the site, if the contributing drainage area is known. However, since all types of drainage basins were used to develop the curves, extreme peak-discharge estimates for sites in which there have been significant effects from manmade modification of streamflow may be obtained if caution is exercised to recognize the limitations of such estimates.

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Supplemental Information

peak discharges and flood frequency estimates for selected streamflow-gaging stations with at least

Station name	Type of station (CONT/CSG)	Documented extreme peak discharge			Potential extreme peak discharge (ft ³ /s) (table 4)
		Type of record (H/I/N/R/U)	Date	Discharge (ft ³ /s)	
Equah, Okla.	CSG	N	06/08/74	5,000	20,000
quah, Okla.	CONT	HN	01/00/16	112,000	219,000
		N	05/10/50	150,000 ^{a,b}	
Is, Ark.	CONT	N	11/18/85	20,900	74,600
la.	CONT	N	05/03/90	50,600	155,000
Okla. ⁷	CONT	N	05/11/50	180,000 ^a	256,000
		R	06/09/57	18,100	
Okla.	CONT	HN	02/00/38	19,300	141,000
		N	05/10/43	42,000 ^a	
as, Okla.	CSG	N	05/23/81	6,270	26,200
ar Hydro, Okla.	CSG	N	09/21/65	1,050	8,230
eport, Okla.	CONT	N	06/23/48	150,000	287,000
		R	05/17/82	86,100	
e, Okla.	CSG	N	10/20/83	3,000	41,500
y near Newcastle, Okla.	CSG	N	04/14/65	1,460	18,800
ole, Okla.	CONT	R	09/22/65	35,500	536,000
l, Okla.	CONT	R	05/29/87	102,000	536,000
chard, Okla.	CSG	N	11/20/63	887	7,820
ell, Okla.	CONT	N	10/20/83	67,700 ^g	136,000
ear Asher, Okla.	CSG	N	05/13/68	2,000	13,600
, Okla.	CSG	N	10/08/70	2,600	13,500
nderbird near Norman, Okla. ⁸	CONT	N	05/25/57	34,600 ^a	146,000
		R	05/10/90	1,450	
eh, Okla.	CONT	HN	06/00/32	60,000 ^a	175,000
		N	05/25/57	32,400	
		R	05/03/90	14,000 ^h	
a, Okla.	CONT	HN	06/00/39	33,000	213,000
		N	05/11/50	44,600 ^a	
		R	05/01/85	18,500	
near Atwood, Okla.	CSG	N	10/08/70	1,470	4,610
n, Okla.	CONT	HN	08/07/06	128,000	548,000
		HN	05/31/37	135,000	
		N	05/11/50	174,000	
		R	05/29/87	154,000	
on, Okla.	CSG	N	05/13/68	5,460	33,100
, Okla.	CSG	N	03/27/77	18,000	39,500
s, Okla.	CONT	HN	02/18/38	70,000 ^a	190,000
		N	05/11/43	62,000	
on, Okla. ⁹	CONT	HN	06/00/37	28,600	192,000
		N	06/15/64	55,400	
		I	09/15/88	3,410	
lt, Okla.	CSG	N	08/19/65	1,900	52,800
uymon, Okla. ⁹	CONT	I	06/20/82	5,800	174,000
ardesty, Okla. ⁹	CONT	N	06/25/47	21,500	176,000
earman, Tex.	CONT	HN	09/04/38	34,000 ^a	185,000
		N	10/07/46	21,200	

8 years of annual peak-discharge data from unregulated, regulated, and urban basins within and

Site number (fig. 1)	Contributing drainage area (mi ²)	Latitude	Longitude	Hydrologic region (E/W)	Type basin (N/I/R/U)	LPIII Peak discharge frequency (ft ³ /s)		
						2 yr	5 yr	10 yr
131	3.59	0355837	0945524	E	N	536	1,760	3,190
132	959	0355522	0945524	E	N	19,800	39,100	54,900
133	40.6	0355248	0942911	E	N	7,110	14,000	19,400
134	307	0355516	0945018	E	N	15,200	26,300	34,300
135	1,626	0353423	0950407	E	N	31,300	64,300	94,300
					R	8,180	11,900	14,300
136	227	0353318	0951828	E	N	9,170	23,700	39,200
137	10.4	0354808	0984715	W	N	794	2,170	3,690
138	2.31	0353210	0982850	W	N	304	539	740
139	20,475	0353237	0981903	W	N	26,200	45,300	60,000
					R	16,200	31,400	43,800
140	11.2	0351728	0974510	E	N	1,260	2,130	2,800
141	3.32	0351727	0973720	E	N	710	1,180	1,530
142	21,110	0350455	0972252	E	R	15,700	24,100	30,800
143	21,138	0350050	0972050	E	R	21,400	43,200	62,700
144	1.26	0350720	0974210	E	N	378	666	900
145	202	0345956	0972200	E	N	8,750	16,900	24,500
146	2.28	0345909	0965848	E	N	400	746	1,070
147	2.26	0345410	0962320	E	N	660	1,210	1,690
148	257	0351318	0971249	E	N	5,300	8,500	11,200
					R	667	960	1,120
149	456	0351021	0965554	E	N	9,200	16,800	24,000
					R	5,040	7,230	8,800
150	865	0345902	0963301	E	N	15,200	26,900	36,700
					R	7,820	11,900	14,600
151	0.72	0345710	0962040	E	N	300	590	800
152	23,151	0345840	0961436	E	N	60,400	94,700	121,000
					R	53,300	88,900	114,000
153	7.40	0351110	0960420	E	N	1,650	3,030	4,200
154	9.99	0344740	0952050	E	N	4,150	7,940	11,200
155	588	0345900	0953700	E	N	11,600	20,500	28,300
156	1,175	0364317	1012921	W	N	8,580	21,400	33,100
					I	181	1,160	2,500
157	31.0	0363323	1024710	W	N	131	706	1,690
158	725	0363419	1012252	W	I	223	979	1,900
159	767	0363838	1011238	W	N	2,720	7,460	12,300
160	960	0361208	1011820	W	N	2,400	6,510	11,200

peak discharges and flood frequency estimates for selected streamflow-gaging stations with at least

Station name	Type of station (CONT/CSG)	Documented extreme peak discharge			Potential extreme peak discharge (ft ³ /s) (table 4)
		Type of record (H/I/N/R/U)	Date	Discharge (ft ³ /s)	
		R	10/20/83	18,100 ^g	
ve Henrietta, Tex.	CONT	R	05/01/66	7,630	224,000
		R	05/03/90	14,200 ^h	
River near Henrietta, Tex.	CONT	N	10/13/81	32,500 ^k	131,000
kla.	CONT	N	06/06/41	197,000	546,000
		R	06/07/95	236,000	
tary near Loco, Okla.	CSG	N	06/22/65	2,100	10,600
y, Okla.	CONT	HN	05/00/57	30,000	188,000
		N	05/03/90	49,600	
ilson, Okla.	CSG	N	04/12/67	2,300	28,200
		HN	10/00/81	6,040	
lle, Tex.	CONT	N	06/09/41	168,000	559,000
		R	05/31/87	265,000	
illan, Okla.	CSG	N	05/23/75	1,380	17,100
		HN	10/00/81	1,980 ^k	
Okla.	CSG	N	10/14/81	14,100 ^k	42,900
enne, Okla.	CONT	HN	04/03/34	52,000	178,000
		N	04/29/54	69,800	
		R	04/22/90	7,250	
6A near Cheyenne, Okla. ¹⁷	CONT	N	05/26/59	2,710	23,300
6 near Cheyenne, Okla. ¹⁷	CONT	N	05/23/54	18,900	41,700
4 near Cheyenne, Okla. ¹⁷	CONT	N	04/18/57	1,160	4,090
7 near Cheyenne, Okla. ¹⁷	CONT	N	04/29/54	6,030	25,700
erlin, Okla.	CONT	R	04/30/54	5,710	63,200
0A near Elk City, Okla. ¹⁷	CONT	N	08/16/68	1,700	9,910
near Elk City, Okla. ¹⁷	CONT	N	05/03/57	1,870	18,500
near Elk City, Okla. ¹⁷	CONT	N	08/16/68	2,850	12,600
near Elk City, Okla. ¹⁷	CONT	N	04/18/57	1,780	2,580
near Elk City, Okla. ¹⁷	CONT	N	06/08/71	2,420	11,600
heyenne, Okla.	CONT	R	04/30/54	6,360	85,200
near Cheyenne, Okla. ¹⁷	CONT	N	04/18/57	4,280	15,900
mon, Okla.	CONT	H	04/03/34	167,000 ^l	197,000
		R	05/17/82	6,000	
Okla.	CONT	N	04/19/57	14,000	201,000
		R	08/26/69	3,010	
on, Okla.	CONT	HN	04/03/34	90,000	210,000
		N	05/16/51	66,800 ^e	
		R	09/15/96	10,800	
ie, Okla.	CONT	N	05/18/49	50,000	227,000
		R	10/20/83	40,600	
Okla.	CONT	R	06/04/95	12,000	101,000
Okla.	CONT	N	05/20/77	7,000	67,600
obb, Okla.	CONT	HN	06/15/37	51,000	138,000
		N	05/17/49	35,000 ^a	
		R	06/23/87	1,280	
ko, Okla.	CONT	N	05/25/03	29,000	326,000

8 years of annual peak-discharge data from unregulated, regulated, and urban basins within and

Site number (fig. 1)	Contributing drainage area (mi ²)	Latitude	Longitude	Hydrologic region (E/W)	Type basin (N/I/R/U)	LPIII Peak discharge (ft ³ /s)		
						2 yr	5 yr	10 yr
						1,280	3,260	5,030
256	1,037	0334936	0981423	E	R			
257	178	0334846	0980505	E	N	1,720	5,360	9,800
258	22,787	0335243	0975603	E	R	45,200	85,300	121,000
259	1.74	0341840	0973400	E	N	483	1,080	1,670
260	572	0340015	0973400	E	N	6,260	16,200	27,300
261	5.74	0340810	0972520	E	N	1,950	2,570	3,020
262	24,846	0334340	0970935	E	R	49,600	88,800	120,000
263	2.97	0340600	0965835	E	N	760	1,100	1,360
264	12.0	0335954	0964935	E	N	2,870	5,430	7,740
265	794	0353735	0994005	W	N	5,500	15,400	26,900
					R	696	2,010	3,540
266	8.78	0352810	0994010	W	N	438	1,060	1,720
267	20.3	0352840	0993610	W	R	1,670	3,910	6,340
268	1.02	0352840	0993610	W	N	290	681	1,070
269	10.1	0353030	0993640	W	N	960	2,370	3,800
270	44.9	0353026	0993327	W	R	670	1,540	2,440
271	2.87	0352800	0993320	W	N	742	1,250	1,640
272	6.46	0352910	0993010	W	R	592	1,210	1,730
273	3.89	0352930	0992920	W	R	977	1,750	2,200
274	0.62	0353040	0993040	W	N	349	684	970
275	3.50	0352940	0993200	W	N	813	1,530	2,140
276	87.1	0353310	0993150	W	R	1,250	2,520	3,500
277	5.33	0353400	0993010	W	N	994	2,320	3,640
278	1,387	0353923	0991821	W	R	1,040	2,240	3,290
279	1,551	0353220	0991010	W	R	864	1,450	1,800
280	1,977	0353151	0985800	W	N	7,800	18,200	29,200
					R	2,090	4,090	6,000
281	3,129	0350702	0983349	W	N	9,210	17,000	23,600
					R	5,740	12,000	18,500
282	132	0351726	0983538	W	R	2,060	4,490	7,000
283	52.0	0351727	0983144	W	N	707	1,850	3,100
284	307	0350837	0982633	W	N	4,420	10,500	16,900
					R	535	1,020	1,340
285	3,656	0350503	0981435	E	N	8,720	18,300	27,400

peak discharges and flood frequency estimates for selected streamflow-gaging stations with at least

ation name	Type of station (CONT/CSG)	Documented extreme peak discharge			Potential extreme peak discharge (ft ³ /s) (table 4)
		Type of record (H/I/N/R/U)	Date	Discharge (ft ³ /s)	
een, Ark. ²⁵	CSG	HN	08/27/47	110,000 ^{a,b}	132,000
		N	12/10/71	71,000	
		R	05/17/82	6,320	
een, Ark. , Ark.	CSG	N	05/13/68	6,240	30,300
	CONT	HN	08/00/15	124,000 ^a	
		N	03/30/45	120,000	
		R	12/10/71	65,100	
remean, Ark.	CSG	N	12/02/82	3,800	40,700
ervoorst, Ark.	CONT	HN	05/06/61	48,000	102,000
		N	12/02/82	32,000	
ueen, Ark. ²⁵	CSG	N	05/13/68	122,000	162,000
		R	07/02/83	22,600	
ear Lockesburg, Ark.	CSG	N	12/26/82	719	4,090
ata, Tex.	CONT	N	12/10/71	20,400	93,000

8 years of annual peak-discharge data from unregulated, regulated, and urban basins within and

Site number (fig. 1)	Contributing drainage area (mi ²)	Latitude	Longitude	Hydrologic region (E/W)	Type basin (N/I/R/U)	LPIII Peak discharge f		
						2 yr	5 yr	10 yr
345	182	0340251	0942447	E	N	15,700	31,200	45,300
						2,300	3,370	4,290
346	6.41	0340244	0941813	E	N	961	2,400	3,840
347	2,662	0335510	0942315	E	N	46,400	71,300	89,200
					R	25,800	35,300	41,500
348	10.7	0334513	0942328	E	N	1,540	2,650	3,490
					N	14,600	25,400	33,800
350	360	0340245	0941242	E	N	27,800	46,600	61,800
					R	8,080	13,200	16,800
351	0.64	0335804	0941125	E	N	189	337	460
					N	4,740	7,580	9,800

²¹ Continuous-record gage prior to WY 1969

²² Historical record length assumed to start from same year as that for nearby station 07336500 for unregulated streamflow period

²³ Historical record length assumed to start from same year as that for nearby station 07335500 for regulated streamflow period

²⁴ Frequency analysis includes streamflow record from nearby station 07338000, not shown in table

²⁵ Continuous-record gage prior to WY 1980

990)

from nearby station 07161000

from nearby station 07174000

or nearby station 07188000

same year as that for nearby station 07196500 for unregulated streamflow period

or nearby station 07230500 for unregulated streamflow period

in Wahl and Tortorelli (1997)

or nearby station 07241000

or nearby station 07249400

., prior to WY 1993

Ark., prior to WY 1970

same year as that for nearby station 07299850 for unregulated streamflow period

from nearby station 07302000

or nearby station 07313500

water retarding structure

from nearby station 07327500, not shown in table

from nearby station 07332000, not shown in table

Peak discharges for selected indirect measurement sites without streamflow-gaging stations and

feet per second; mi², square miles; E, sites east of 98 degrees 15 minutes longitude;

Indirect measurement site name	Documented extreme peak discharges			Potential extreme peak discharge (ft ³ /s) (table 4)
	Type basin (N/R/U)	Date	Discharge (ft ³ /s)	
Edmond, Okla.	N	05/16/57	14,600	50,600
Lawrence, Okla.	N	05/12/50	442,000	856,000
	R	05/27/57	544,000	
	R	04/09/65	110,000	
	N	05/16/51	7,700	127,000
near Arapaho, Okla. ¹	N	05/16/51	2,990	7,780
near Arapaho, Okla.	N	05/16/51	1,590	3,320
Okla.	N	05/19/43	29,000	135,000
Archer, Okla. ²	N	10/10/73	10,300	57,100
Medford, Okla.	N	05/19/43	22,000	190,000
Creek near Page, Okla.	N	11/05/94	19,600	95,800
	N	4/19/76	6,170	43,000
Hefner near Oklahoma City, Okla.	U	06/16/55	1,070	9,910
City, Okla.	U	05/20/77	1,640	10,000
Road at Enid, Okla. ²	U	10/10/73	8,730	31,800
al below Rupe Ave in Enid, Okla. ²	N	10/10/73	13,200	43,500
Okla.	N	10/20/84	4,220	93,000
Street at Oklahoma City, Okla. ³	U	05/08/93	12,000	24,800
Street at Oklahoma City, Okla. ³	U	05/08/93	10,000	18,400
Street at Oklahoma City, Okla. ³	U	05/08/93	8,000	13,300
36th East Ave Bridge at Tulsa, Okla. ⁴	U	05/27/84	5,910	13,000
ity, Mo. ¹	N	05/18/43	23,000	99,500
castle, Okla.	N	05/04/41	200,000	535,000
f Wauhatchie, Okla.	N	05/00/43	17,000	96,400
Geary, Okla.	N	09/21/65	6,150	28,500
e Drive, The Village, Okla.	U	05/20/77	2,800	11,500
t.	U	07/23/63	4,000	35,500
Okla. ⁵	N	10/00/81	3,930	35,800
near Dewar, Okla.	N	05/16/45	6,820	66,600
st Ave at Tulsa, Okla. ⁴	U	05/27/84	4,160	12,800
ate 44 and 129th East Ave at Tulsa, Okla. ⁴	U	05/27/84	5,040	13,500
, Okla. ^{1, 6}	N	04/14/45	11,000	39,500
Guthrie, Okla. ⁷	N	05/19/49	44,000	161,000
Navina, Okla.	UR	09/13/89	40,300	144,000
ne, Okla. ¹	N	05/10/50	43,200	90,800
Mountain View, Okla.	N	05/18/49	5,800	20,600

streamflow-gaging stations with short periods of record in basins within Oklahoma

W, sites west of 98 degrees 15 minutes longitude; Ck, Creek; Ave, Avenue; SW, southwest; Trib, Tributary; NW, northwest

Site number (fig. 2)	Contributing drainage area (mi ²)	Latitude	Longitude	Hydrologic region (E/W)
353	28.7	0364521	0990539	W
354	125,516	0352058	0944816	E
355	243	0353450	0990235	W
356	2.16	0353450	0990131	W
357	0.81	0353818	0985918	W
358	195	0363753	0961429	E
359	22.2	0362330	0974030	E
360	590	0362023	0964232	E
361	74.4	0345246	0943040	E
362	12.1	0350226	0953421	E
363	1.62	0353233	0973546	E
364	1.64	0353226	0973556	E
365	6.91	0362325	0975440	E
366	12.4	0362225	0975400	E
367	69.1	0350121	0945639	E
368	4.74	0352607	0973253	E
369	3.24	0352514	0973259	E
370	2.24	0352422	0973315	E
371	2.17	0360720	0944930	E
372	82.0	0364043	0943537	E
373	20,962	0351803	0973554	E
374	75.4	0354753	0945118	E
375	11.8	0353255	0981550	W
376	1.90	0353356	0973326	E
377	8.18	0361224	0955448	E
378	8.50	0342706	0961356	E
379	31.4	0352815	0955414	E
380	2.14	0360948	0955000	E
381	2.26	0360932	0954956	E
382	10.0	0351303	0963048	E
383	353	0355013	0972625	E
384	247	0354636	0973245	E
385	65.0	0342420	0975630	E
386	7.45	0350542	0984543	W

peak discharges for selected indirect measurement sites without streamflow-gaging stations and

ion name and location	Documented extreme peak discharges			Potential extreme peak discharge (ft ³ /s) (table 4)
	Type basin (N/R/U)	Date	Discharge (ft ³ /s)	
Okla.	N	05/17/57	12,600	85,000
roll, Okla.	N	04/29/54	8,410	55,000
trong City, Okla. ⁸	N	4/3-4/34	17,000	69,900
pendence, Oklahoma City, Okla.	U	11/02/74	3,940	22,000
reet Culvert, Oklahoma City, Okla.	U	11/02/74	4,100	28,500
e., Oklahoma City, Okla.	U	05/20/77	4,130	17,200
kla. ¹	N	05/18/43	42,000	227,000
ity, Okla. ¹	N	05/16/51	46,400	86,600
ek, Okla. ²	N	10/11/73	36,400	83,900
Okla.	N	06/00/48	70,000	138,000
r Custer City, Okla. ¹	N	05/16/51	7,030	19,100
o, Okla. ¹	N	06/22/48	31,000	134,000
anucka, Okla. ⁵	N	10/00/11	13,700	78,700
ary at Mohawk Road at Tulsa, Okla. ⁴	U	05/27/84	2,920	15,900
Okla. ¹	N	10/04/55	8,170	26,000
e, Okla.	N	05/18/55	5,320	50,700
t, Okla. ¹	N	05/00/43	20,000	122,000
armen, Okla. ¹	N	05/16/57	31,800	138,000
ear Moorewood, Okla. ⁸	N	04/03-04/34	6,300	12,900
er Creek near Moorewood, Okla. ⁸	N	04/03-04/34	54,500	61,300
reek near Elk City, Okla.	N	04/29/54	1,340	14,600
Bowlegs, Okla. ^{1, 6, 9}	N	04/14/45	3,000	5,640
r, Okla.	N	10/31/72	968	7,640
ear Boise City, Okla.	N	08/21/65	2,700	15,500
Reno, Okla. ¹	N	11/19/53	6,390	35,900
gs, Okla.	R	05/10/50	20,100	101,000
rdesty, Okla. ¹	R	05/16/55	22,100	95,400
ukomis, Okla.	N	05/16/57	16,500	66,300
et in Tulsa, Okla.	N	05/27/84	6,040	27,300
etta, Okla. ⁵	N	10/13-14/81	68,100	112,000
linsville, Okla.	N	05/22/53	2,540	36,000
Okla.	N	05/16/57	6,800	61,600
ssa, Okla. ¹	N	05/18/43	15,800	45,200
reet South at Tulsa, Okla. ⁴	U	05/27/84	23,000	47,200
er, Okla.	R	05/10/50	459	11,200
, Okla.	N	05/08/50	11,200	51,200

streamflow-gaging stations with short periods of record in basins within Oklahoma—Continued

Site number (fig. 2)	Contributing drainage area (mi ²)	Latitude	Longitude	Hydrologic region (E/W)
387	55.0	0345303	0973140	E
388	33.8	0354654	0994243	W
389	56.0	0354056	0993750	W
390	4.02	0353018	0973428	E
391	5.83	0353041	0973323	E
392	2.98	0353006	0973458	E
393	1,093	0354111	0963943	E
394	90.2	0354155	0984940	W
395	53.1	0364808	0973010	E
396	308	0353234	0983206	W
397	6.74	0354112	0985036	W
398	280	0353233	0983518	W
399	45.8	0342430	0962515	E
400	2.74	0361158	0955815	E
401	10.3	0352830	0990245	W
402	17.0	0342144	0980108	E
403	144	0354326	0964439	E
404	306	0363444	0983037	W
405	4.00	0354509	0992339	W
406	42.0	0354333	0992109	W
407	4.73	0353129	0993156	W
408	0.89	0351440	0963354	E
409	1.23	0340633	0945542	E
410	5.15	0364620	1024816	W
411	8.51	0353220	0975249	E
412	87.0	0353653	0951023	E
413	116	0363259	1010948	W
414	31.0	0361513	0974802	E
415	5.45	0360154	0955019	E
416	116	0340046	0970459	E
417	8.55	0362316	0955304	E
418	26.4	0361446	0981145	E
419	13.4	0364734	0945212	E
420	14.6	0360339	0955801	E
421	1.85	0352930	0952040	E
422	17.4	0362328	0980355	E

peak discharges for selected indirect measurement sites without streamflow-gaging stations and

ion name and location	Documented extreme peak discharges			Potential extreme peak discharge (ft ³ /s) (table 4)
	Type basin (N/ R/U)	Date	Discharge (ft ³ /s)	
Pump Station near Jennings, Okla. ¹ en, Okla. ^{10, 11} kla. ^{10, 11}	N	09/04/40	43,600	80,700
	R	05/00/90	430,000	627,000
	N	05/00/90	120,000	259,000
Farris, Okla. ^{10, 11} ar Dougherty, Okla. ^{10, 12} , Okla. ^{10, 11} , Okla. ^{10, 11} on, Tex. ^{10, 11}	N	05/00/90	32,000	131,000
	R	10/08–09/70	80,000	116,000
	N	05/00/90	114,000	150,000
	N	05/00/90	70,000	255,000
	R	05/00/90	300,000	608,000
ka, Okla. ⁶ Okla. ^{10, 11} y, Okla. Avenue at Oklahoma City, Okla. ³ Street at Oklahoma City, Okla. ³	R	04/14/45	9,700	49,300
	N	05/00/90	100,000	222,000
	N	06/22/48	2,410	3,440
	U	05/08/93	15,000	44,000
	U	05/08/93	10,000	35,700
th Street at Oklahoma City, Okla. ³ th Street at Oklahoma City, Okla. ³ th Street at Oklahoma City, Okla. ³ st Street at Oklahoma City, Okla. ³ th Street in Oklahoma City, Okla.	U	05/08/93	15,000	46,100
	U	05/08/93	15,000	45,500
	U	05/08/93	10,000	41,800
	U	05/08/93	10,000	35,000
	U	05/29/70	4,840	46,700
th Street in Oklahoma City, Okla. ew, Okla. omas, Okla. ¹ n, Okla. ¹ , Okla.	U	05/29/70	2,720	21,700
	N	05/09/43	17,000	93,400
	N	05/16/51	6,230	15,100
	N	05/16/57	17,300	61,300
	N	03/27/77	36,000	130,000
etwater, Okla. nid, Okla. rk at Tulsa, Okla. ⁴ la. ¹³ Okla. ⁵	N	04/29/54	1,210	32,100
	U	07/29/50	5,830	43,400
	U	05/27/84	12,300	14,700
	N	10/08/70	15,300	102,000
	N	10/16/81	29,800	173,000
o, Okla. mmon, Okla. ⁸ id, Okla. ² near Blanchard, Okla. ulgee, Okla. ⁶	N	09/20/65	1,740	12,500
	N	04/03–04/34	36,000	61,300
	U	10/10/73	10,200	32,500
	N	05/28/49	36,400	81,400
	R	04/14/45	11,000	74,200
k at Lahoma Road at Enid, Okla. ² lt (at Narrows), Okla. Roosevelt, Okla.	U	10/10/73	1,660	2,380
	N	06/05/53	16,300	64,600
	N	06/05/53	2,670	11,500

streamflow-gaging stations with short periods of record in basins within Oklahoma—Continued

Site number (fig. 2)	Contributing drainage area (mi ²)	Latitude	Longitude	Hydrologic region (E/W)
423	48.3	0360853	0963533	E
424	37,822	0351825	0952145	E
425	1,709	0340042	0952249	E
426	178	0341852	0955230	E
427	126	0342550	0970150	E
428	275	0343745	0952103	E
429	1,610	0353548	0950257	E
430	33,784	0334905	0963420	E
431	16.0	0351037	0963137	E
432	993	0345612	0944310	E
433	0.84	0353641	0982540	W
434	12.7	0352541	0973046	E
435	8.42	0352500	0973100	E
436	13.9	0352621	0973043	E
437	13.6	0352606	0973050	E
438	11.4	0352514	0973050	E
439	8.12	0352451	0973105	E
440	14.3	0352644	0973033	E
441	3.96	0352328	0973136	E
442	69.9	0354822	0962930	E
443	4.96	0354516	0984839	W
444	42.0	0364244	0990708	W
445	176	0341854	0955230	E
446	14.0	0352944	0995930	W
447	12.3	0362327	0975345	E
448	2.50	0360900	0955322	E
449	89.2	0341535	0964837	E
450	445	0342323	0960712	E
451	3.84	0342700	0993210	W
452	42.0	0353831	0992754	W
453	7.17	0362400	0975224	E
454	49.2	0350951	0973651	E
455	40.1	0353717	0960342	E
456	0.38	0362327	0975340	E
457	47.0	0344910	0990137	W
458	3.45	0345156	0990131	W

Peak discharges for selected indirect measurement sites without streamflow-gaging stations and

Station name and location	Documented extreme peak discharges			Potential extreme peak discharge (ft ³ /s) (table 4)
	Type basin (N/R/U)	Date	Discharge (ft ³ /s)	
Okla.	N	05/10/50	4,740	28,600
Lawton, Okla. ¹	N	05/19/43	5,500	33,400
Okmulgee, Okla.	N	09/03/40	51,600	141,000
near Jefferson, Okla. ²	N	10/11/73	24,300	130,000
Okla. Cache, Okla. ¹⁴	N	08/27/71	23,000	106,000
Okla. ¹⁵	R	05/03/90	74,600	262,000
near Mountain View, Okla. ¹	N	05/17/49	38,000	139,000
tributary West of Mountain View, Okla.	N	05/17/49	3,300	9,100
Okla. ^{1, 9}	N	09/04/40	32,400	50,800
near Enid, Okla. ²	N	10/11/73	55,700	87,600
Okla. ⁶	N	04/14/45	5,600	25,800
Okla. ¹²	N	10/08/70	27,800	73,400
near Enid, near Sulphur, Okla. ^{10, 13}	N	10/08/70	6,350	7,350
Okla. ¹	N	04/29/54	53,700	77,600
near Enid, Okla. ²	N	10/10/73	12,400	48,600
near Cheyenne, Okla. ⁸	N	04/04/34	52,100	56,800
Okla. ²	N	10/10/73	24,100	51,100
near Enid, near Enid, Okla. ²	U	10/10/73	60,600	94,100
Okla. ¹	U	05/16/57	3,750	20,300
near Guymon, Okla.	N	04/30/82	81	972
near Lawton, Okla. ¹	R	04/19/41	86,400	166,000
near Lawton, Okla.	U	11/02/74	3,440	30,400
near Highway, Oklahoma City, Okla.	U	11/02/74	2,060	13,600
near Dam, Oklahoma City, Okla.	U	11/02/74	3,000	32,400
Okla.	N	05/28/70	927	3,780
near Enid, Okla.	N	05/08/93	4,500	69,200
near Enid, Okla.	N	05/10/50	41,200	112,000
near Enid, Okla.	N	05/08/93	2,240	57,000
near Enid, Okla.	N	05/18/43	9,700	71,800
near Enid, Okla. ¹³	N	10/08/70	16,500	17,300
near Highway 169 at Tulsa, Okla. ⁴	U	05/26-27/84	9,540	19,700
near Enid, Okla. ²	N	10/11/73	52,700	166,000
near Washita River near Arapaho, Okla.	N	05/16/51	10,900	35,300
near Enid, Okla.	U	05/08/93	10,000	18,400
near Enid, Okla. ³	U	05/08/93	6,000	15,900
near Enid, near Beaver Creek, Cache, Okla. ¹⁴	N	08/27-28/77	933	1,870

streamflow-gaging stations with short periods of record in basins within Oklahoma— Continued

Site number (fig. 2)	Contributing drainage area (mi ²)	Latitude	Longitude	Hydrologic region (E/W)
459	5.85	0345135	0971555	E
460	7.50	0364158	0960246	E
461	229	0355719	0960707	E
462	173	0364325	0974745	E
463	148	0343125	0983757	W
464	1,767	0350956	0943910	E
465	316	0350554	0984334	W
466	2.60	0350515	0984704	W
467	17.1	0361736	0963409	E
468	59.4	0363130	0973955	E
469	5.00	0351022	0964011	E
470	39.2	0343056	0965806	E
471	1.18	0343143	0965755	E
472	69.6	0354045	0995100	W
473	15.5	0362327	0975913	E
474	36.0	0353633	0994043	W
475	17.3	0362451	0974916	E
476	71.1	0362135	0974815	E
477	3.66	0362231	0975324	E
478	0.26	0364006	1012954	W
479	386	0362301	0950313	E
480	6.42	0353322	0973718	E
481	2.28	0353305	0973717	E
482	7.10	0353431	0973724	E
483	0.93	0352425	0993305	W
484	34.4	0351121	0981054	E
485	116	0360853	0950929	E
486	22.1	0351121	0980735	E
487	37.3	0365640	0945114	E
488	3.00	0343003	0965725	E
489	3.53	0360906	0955136	E
490	385	0360338	0975550	E
491	16.1	0353403	0985411	W
492	3.23	0352606	0973358	E
493	2.73	0352514	0973405	E
494	0.45	0343751	0983323	W

Peak discharges for selected indirect measurement sites without streamflow-gaging stations and

Station name and location	Documented extreme peak discharges			Potential extreme peak discharge (ft ³ /s) (table 4)
	Type basin (N/R/U)	Date	Discharge (ft ³ /s)	
Master Ck near Moorewood, Okla. ⁸	N	04/03-04/34	18,100	41,000
Peeyah, Okla.	N	05/21/43	138,000	345,000
Payville, Okla. ⁵	N	10/13-14/81	29,600	156,000
Grove, Okla. ¹	N	05/17/57	63,000	118,000
ia, Okla.	N	05/18/43	4,200	42,900
ll, Okla. ⁸	N	04/04/34	65,000	213,000
Arapaho, Okla. ⁸	N	04/03-04/34	16,600	111,000
Creek near Cache, Okla. ¹⁴	N	08/27-28/77	6,370	14,400
near Moorewood, Okla. ⁸	N	04/03-04/34	9,400	48,700
er Creek near Moorewood, Okla. ⁸	N	04/03-04/34	34,300	88,600
er Creek near Moorewood, Okla. ⁸	N	04/03-04/34	67,400	92,400
ache, Okla. ¹⁴	N	08/28/77	13,600	73,900
axon, Okla. ¹⁴	N	08/28/77	45,700	135,000
Okla. ^{1, 6}	N	04/14/45	88,000	96,200
lsdale, Okla. ²	N	10/10/73	6,280	26,100
kins, Okla.	N	05/20/57	23,000	72,400
nd Creek, Okla. ²	N	10/11/73	102,000	112,000
ma, Okla.	N	05/19/55	19,300	111,000
Okla. ¹	N	05/10/50	5,890	21,300
, Okla.	N	05/20/77	12,000	50,800

streamflow-gaging stations with short periods of record in basins within Oklahoma—Continued

Site number (fig. 2)	Contributing drainage area (mi ²)	Latitude	Longitude	Hydrologic region (E/W)
495	20.0	0354148	0991833	W
496	4,402	0362330	0954015	E
497	314	0335630	0971820	E
498	133	0340911	0971945	E
499	12.0	0365546	0943905	E
500	2,160	0352058	0985126	W
501	112	0353819	0990501	W
502	4.67	0343801	0983622	W
503	27.0	0354509	0992423	W
504	61.0	0353924	0971820	E
505	107	0354215	0992236	W
506	63.1	0343609	0983757	W
507	285	0342746	0983413	W
508	75.0	0351026	0963633	E
509	5.08	0363305	0975440	E
510	38.1	0355907	0970707	E
511	116	0363723	0974736	E
512	112	0342906	0974144	E
513	3.87	0342841	0975708	E
514	28.9	0351400	0982757	W

(1990)
(1)
tion Service (1970)